$\Delta$
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## GATE 2017 <br> Civil Engineering

## Questions with Detailed Solutions

## AFTERNOON SESSION

## Section: Civil Engineering

1. Let the characteristic strength be defined as that value, below which not more than $50 \%$ of the results are expected to fall. Assuming a standard deviation of 4 MPa , the target mean strength (in MPa ) to be considered in the mix design of a M25 concrete would be
(A) 18.42
(B) 21.00
(C) 25.00
(D) 31.58
2. Ans: (C)

Sol:
$\mathrm{M} 25, \mathrm{f}_{\mathrm{ck}}=25 \mathrm{~N} / \mathrm{mm}^{2}$

$\sigma=4 \mathrm{MPa}$
If $50 \%$ of test result are expected to fall then target mean strength $f_{m}=f_{c k}$
$\mathrm{f}_{\mathrm{m}}=25 \mathrm{~N} / \mathrm{mm}^{2}$
02. Let $\mathrm{W}=\mathrm{f}(\mathrm{x}, \mathrm{y})$, where x and y are functions of t . Then, according to the chain rule, $\frac{\mathrm{dw}}{\mathrm{dt}}$ is equal to
(A) $\frac{\mathrm{dw}}{\mathrm{dx}} \frac{\mathrm{dx}}{\mathrm{dt}}+\frac{\mathrm{dw}}{\mathrm{dy}} \frac{\mathrm{dt}}{\mathrm{dt}}$
(B) $\frac{\partial w}{\partial x} \frac{\partial x}{\partial t}+\frac{\partial w}{\partial y} \frac{\partial y}{\partial t}$
(C) $\frac{\partial w}{\partial x} \frac{d x}{d t}+\frac{\partial w}{\partial y} \frac{d y}{d t}$
(D) $\frac{d w}{d x} \frac{\partial x}{\partial t}+\frac{d w}{d y} \frac{\partial y}{\partial t}$
02. Ans: (C)

Sol: $\quad$ Let $w=f(x, y)$, where $x \& y$ are functions of $t$
By chain rule, $\frac{d w}{d t}=\frac{\partial w}{\partial x} \frac{d x}{d t}+\frac{\partial w}{\partial y} \frac{d y}{d t}$
Option (C) is correct
03. A sheet pile has an embedment depth of 12 m in a homogeneous soil stratum. The coefficient of permeability of soil is $10^{-6} \mathrm{~m} / \mathrm{s}$. Difference in the water levels between the two sides of the sheet pile is 4 m . The flow net is constructed with five number of flow lines and eleven number of equipotential lines. The quantity of seepage ( $\mathrm{in} \mathrm{cm}^{3} / \mathrm{s}$ per m . up to one decimal place) under the sheet pile is $\qquad$
03. Ans: $\mathbf{1 . 6}$

Sol:

$\mathrm{K}=10^{-6} \mathrm{~m} / \mathrm{s}, \mathrm{H}=4 \mathrm{~m}$
Number of flow lines $=5$ [including boundary flow lines]
$\therefore$ Number of flow channels, $\mathrm{N}_{\mathrm{f}}=5-1=4$
Number of equipotential lines $=11$ (including boundary equipotential lines)
$\therefore$ Number of potential drops, $\mathrm{N}_{\mathrm{d}}=11-1=10$

$$
\therefore \mathrm{Q}=\mathrm{kH} \frac{\mathrm{~N}_{\mathrm{f}}}{\mathrm{~N}_{\mathrm{d}}}
$$

$$
\begin{aligned}
& =10^{-6} \times 4 \times \frac{4}{10}=1.6 \times 10^{-6} \mathrm{~m}^{3} / \mathrm{s} / \mathrm{m} \\
& =1.6 \times 10^{-6} \times 10^{6} \mathrm{~cm}^{3} / \mathrm{s} / \mathrm{m} \\
& =1.6 \mathrm{~cm}^{3} / \mathrm{s} / \mathrm{m}
\end{aligned}
$$

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04. Consider the frame shown in the figure.


If the axial and shear deformations in different members of the frame are assumed to be negligible, the reduction in the degree of kinematical indeterminacy would be equal to
(A) 5
(B) 6
(C) 7
(D) 8
04. Ans: (B)

Sol:
$=4 ; j=6 ; m=6$


Members axially deformable
$\mathrm{D}_{\mathrm{K}}=3 \mathrm{j}-\mathrm{r}=3 \times 6-4=14$
Members axially rigid
$\mathrm{D}_{\mathrm{K}}=3 \mathrm{j}-(\mathrm{r}+\mathrm{m})=14-6=8$
Thus, Reduction in kinematic indeterminacy $=14-8=6$
05. As per Noise Pollution (regulation and Control) Rules 2000 of India, the day time noise limit for a residential zone, expressed in $d B(A) L_{e q}$ is
(A) 55
(B) 65
(C) 75
(D) 85
05. Ans: (A)

## Sol:

As per Indian standard, day time noise level at residential area limited to 55 dB .
06. For a broad gauge railway track on a horizontal curve of radius $R$ (in $m$ ), the equilibrium cant $e$ required for a train moving at a speed of V (in km per hour) is
(A) $\mathrm{e}=1.676 \frac{\mathrm{~V}^{2}}{\mathrm{R}}$
(B) $\mathrm{e}=1.315 \frac{\mathrm{~V}^{2}}{\mathrm{R}}$
(C) $e=0.80 \frac{\mathrm{~V}^{2}}{R}$
(D) $\mathrm{e}=0.60 \frac{\mathrm{~V}^{2}}{\mathrm{R}}$
06. Ans: (B)

Sol: The equilibrium cant, $\mathrm{e}=\frac{\mathrm{G} \cdot \mathrm{V}^{2}}{127 \mathrm{R}}$
Where e, G, R ------in metres
V -------- in kmph
For $\mathrm{BG}, \mathrm{G}=1.676 \mathrm{~m}, \quad \mathrm{e}=\frac{\mathrm{GV}^{2}}{1.27 \mathrm{R}}$
( e is in cm )

$$
\begin{aligned}
\therefore \text { For } \mathrm{BG} & \rightarrow \mathrm{e}=1.315 \frac{\mathrm{~V}^{2}}{\mathrm{R}}(\mathrm{e} \text { is in } \mathrm{cm}) \\
\mathrm{MG} & \rightarrow \mathrm{e}=0.8 \frac{\mathrm{~V}^{2}}{\mathrm{R}}(\mathrm{e} \text { is in } \mathrm{cm})
\end{aligned}
$$

$$
\mathrm{NG} \rightarrow \mathrm{e}=0.6 \frac{\mathrm{~V}^{2}}{\mathrm{R}}(\mathrm{e} \text { is in } \mathrm{cm})
$$

Note: In the questions given in the options,
The cant be should have been mentioned in 'cm'. This point is missing in the question.
07. While aligning a hill road with a ruling gradient of $6 \%$, a horizontal curve of radius 50 m is encountered. The grade compensation (in percentage, up to two decimal places) to be provided for this case would be $\qquad$
07. Ans: 1.5

Sol:
Ruling gradient, $\mathrm{G}=6 \%$ ( $>4 \%$ grade compensation is allowed as per 1RC)
Radius of curve, $\mathrm{R}=50 \mathrm{~m}$
Grade compensation, $\mathrm{GC}=\frac{30+\mathrm{R}}{\mathrm{R}}$

$$
\begin{aligned}
& =\frac{30+50}{50}=1.6 \% \\
& =\frac{75}{\mathrm{R}}=\frac{75}{50}=1.5
\end{aligned}
$$

$\therefore$ Use min value of GC $=1.5 \%$
08. The VPI (vertical point of intersection) is 100 m away (when measured along the horizontal) from the VPC (vertical point of curvature). If the vertical curve is parabolic, the length of the curve (in meters and measured along the horizontal) is
08. Ans: 200 m

## Sol: Explanation:

Since a vertical curve is quite flat for a parabolic curve, a line IH drawn from I (point of inter section) parallel to the axis on the parabola bisects the long chord joining $T_{1}$ and $T_{2}$ i.e $T_{1} H=H T_{2}$ (Shown in figure)


Vertical curve bisects the distance between $I$ and $H$, the middle point of $T_{1} T_{2}$ i.e $I M=M H$
Since the curve is so flat, that the length of the vertical curve $\mathrm{T}_{1} \mathrm{MT}_{2}=\mathrm{T}_{1} \mathrm{HT}_{2} \&$

$$
\mathrm{T}_{1} \mathrm{H}=\mathrm{T}_{1} \mathrm{M}=\mathrm{IT}_{1}=\mathrm{IT}_{2}
$$

Given $\mathrm{IT}_{1}=100 \mathrm{~m}=\mathrm{T}_{1} \mathrm{H}$
Length of curve $=\mathrm{MT}_{1}+\mathrm{MT}_{2}=1.00+100=200$
09. A two-faced fair coin has its faces designated as head $(\mathrm{H})$ and tail $(\mathrm{T})$. This coin is tossed three times in succession to record the following outcomes: $\mathrm{H}, \mathrm{H}, \mathrm{H}$. If the coin is tossed one more time, the probability (up to one decimal place) of obtaining H again, given the previous realizations of $\mathrm{H}, \mathrm{H}$ and H , would be $\qquad$
09. Ans: 0.5

Sol: Here, probability remains same for each toss.
$\therefore$ Probability of getting head in the $4^{\text {th }}$ toss $=\frac{1}{2}=0.5$
10. For a construction project, the mean and standard deviation of the completion time are 200 days and 6.1 days, respectively. Assume normal distribution and use the value of standard normal deviate $=1.64$ for the $95 \%$ confidence level. The maximum time required (in days) for the completion of the project would be
10. Ans: 210

## Sol:

$$
\text { Mean }=(\mu)=200 \text { days }
$$

Std. deviation $(\sigma)=6.1$ days
$\mathrm{Z}=1.64$ ( $95 \%$ confidence level)
Max time required to complete the project (due date) $=$ ?

$$
\mathrm{Z}=\frac{\mathrm{X}-\mu}{\sigma}
$$

Where $\mathrm{X}=$ due date

$$
\mu=\text { mean }
$$

$\sigma=$ std. deviation
$1.64=\frac{X-200}{6.1}$
$X=210.004$

$\mu=200{ }^{1.64 \sigma} \stackrel{\rightharpoonup}{\mathrm{X}}=210$
$X \simeq 210$ days
11. Consider a rigid retaining wall with partially submerged cohesionless backfill with a surcharge. Which one of the following diagrams closely represents the Rankine's active earth pressure distribution against this wall?
(A)

(B)

(C)

(D)


## 11. Ans: (B)

## Sol:


12. Given that the scope of the construction work is well-defined with all its drawings, specifications, quantities and estimates, which one of the following types of contract would be most preferred?
(A) EPC contract
(B) Percentage rate contract
(C) Item rate contract
(D) Lump sum contract

## 12. Ans: (D)

# SHORT TERM BATCHES FOR GATE+PSUs -2018 STARTING FROM HYDERABAD <br> 29 ${ }^{\text {th }}$ APRIL 2017 onwards 

## GENERAL STUDIES BATCHES FOR ESE-2018 STARTING FROM <br> HYDERABAD \& DELHI <br> $1^{\text {st }}$ week of July 2017

13. The infiltration capacity of a soil follows the Horton's exponential model, $f=c_{1}+c_{2} e^{-k t}$. During an experiment, the initial infiltration capacity was observed to be $200 \mathrm{~mm} / \mathrm{h}$. After a long time, the infiltration capacity was reduced to $25 \mathrm{~mm} / \mathrm{h}$. If the infiltration capacity after 1 hour was $90 \mathrm{~mm} / \mathrm{h}$, the value of the decay rate constant, $\mathrm{k}\left(\right.$ in $\mathrm{h}^{-1}$, up to two decimal places) is $\qquad$
14. Ans: 0.99

Sol:

$$
\begin{aligned}
& \mathrm{f}_{\mathrm{ct}}=\mathrm{f}_{\mathrm{c}}+\left(\mathrm{f}_{\mathrm{co}}-\mathrm{f}_{\mathrm{c}}\right) \mathrm{e}^{-\mathrm{kt}} \\
& \mathrm{f}=\mathrm{C}_{1}+\mathrm{C}_{2} \mathrm{e}^{-\mathrm{kt}} \\
& \mathrm{f}=25+(200-25) \mathrm{e}^{-\mathrm{kt}} \\
& \mathrm{f}=25+175 \mathrm{e}^{-\mathrm{kt}}=\mathrm{R} \\
& \mathrm{f}_{1}=25+175 \mathrm{e}^{-\mathrm{k} \times 1}=90 \\
& \mathrm{k}=\ln \left[\frac{90-25}{175}\right]=0.99 \mathrm{hr}^{-1}
\end{aligned}
$$

14. The method of orientation used, when the plane table occupies a position not yet located on the map, is called as
(A) Traversing
(B) Radiation
(C) Levelling
(D) Resection
15. Ans: (D)

Sol: Resection is the process of orientation which can be used to locate a point occupied by a plane table as the map with referring to well defined ground stations.
15. In a material under a state of plane strain, a $10 \times 10 \mathrm{~mm}$ square centered at a point gets deformed as shown in the figure.


If the shear strain $\gamma_{\mathrm{xy}}$ at this point is expressed as 0.001 k (in rad), the value of k is
(A) 0.50
(B) 0.25
(C) -0.25
(D) -0.50
15. Ans: (D)

## Sol:

The shear strain $=-0.0005$

$$
\begin{aligned}
\gamma_{\mathrm{xy}} & =0.001(\mathrm{k}) \\
& =0.001 \times(-0.5) \\
\therefore \mathrm{k} & =(-) 0.5
\end{aligned}
$$

16. Following observations have been made for the elevation and temperature to ascertain the stability of the atmosphere

| Elevation (in m) Temperature <br> (in ${ }^{\circ} \mathbf{C}$ ) <br> 10 15.5 <br> 60 15.0 <br> 130 14.3${ }^{2}$ |  |
| :---: | :---: |

The atmosphere is classified as
(A) Stable
(B) Unstable
(C) Neutral
(D) Inverse
16. Ans: (C)

Sol:

$$
\begin{aligned}
& \operatorname{ELR}=\frac{\mathrm{dT}}{\mathrm{dZ}}=\frac{\mathrm{T}_{2}-\mathrm{T}_{1}}{\mathrm{Z}_{2}-\mathrm{Z}_{1}}=\frac{\mathrm{T}_{3}-\mathrm{T}_{2}}{\mathrm{Z}_{3}-\mathrm{Z}_{1}} \times 100 \\
& \frac{15.0-15.5}{60-10} \times 100=\frac{14.3-15}{130-60} \times 100 \\
& \mathrm{ELR}=-1^{\circ} \mathrm{C} / 100 \mathrm{~m} \\
& \mathrm{ALR}=-9.8^{\circ} \mathrm{C} / \mathrm{km} \simeq-10^{\circ} \mathrm{C} / \mathrm{km}=-1{ }^{\circ} \mathrm{C} / 100 \mathrm{~m} \\
& \because \mathrm{ELR}=\mathrm{ALR}
\end{aligned}
$$

$\therefore$ Neutral atmospheric condition.
17. During a storm event in a certain period, the rainfall intensity is $3.5 \mathrm{~cm} /$ hour and the $\phi$-index is 1.5 $\mathrm{cm} /$ hour. The intensity of effective rainfall (in $\mathrm{cm} /$ hour, up to one decimal place) for this period is $\qquad$
17. Ans: 2

Sol: $\quad i=3.5 \mathrm{~cm} / \mathrm{hr}$
$\phi$-Index $=1.5 \mathrm{~cm} / \mathrm{hr}$
Effective rainfall intensify ERI $=(\mathrm{i}-\phi)$

$$
\begin{aligned}
& =(3.5-1.5) \\
& =2 \mathrm{~cm} / \mathrm{hr}
\end{aligned}
$$

18. Let $G$ be the specific gravity of soil solids, $w$ the water content in the soil sample, $\gamma_{\mathrm{w}}$ the unit weight of water, and $\gamma_{d}$ the dry unit weight of the soil. The equation for the zero voids line in a compaction test plot is
(A) $\gamma_{d}=\frac{G \gamma_{w}}{1+G w}$
(B) $\gamma_{d}=\frac{G \gamma_{w}}{G w}$
(C) $\gamma_{d}=\frac{G w}{1+\gamma_{w}}$
(D) $\gamma_{\mathrm{d}}=\frac{\mathrm{Gw}}{1-\gamma_{\mathrm{w}}}$
19. Ans: (A)

Sol: Equation of zero air mids line, $\gamma_{d}=\frac{\gamma_{w} G}{1+\omega \cdot G}$
19. Consider the following simultaneous equations (with $\mathrm{c}_{1}$ and $\mathrm{c}_{2}$ being constants):
$3 \mathrm{x}_{1}+2 \mathrm{x}_{2}=\mathrm{c}_{1}$
$4 \mathrm{x}_{1}+\mathrm{x}_{2}=\mathrm{c}_{2}$
The characteristic equation for these simultaneous equations is
(A) $\lambda^{2}-4 \lambda-5=0$
(B) $\lambda^{2}-4 \lambda+5=0$
(C) $\lambda^{2}+4 \lambda-5=0$
(D) $\lambda^{2}+4 \lambda+5=0$
19. Ans: (A)

Sol: Given $3 \mathrm{x}_{1}+2 \mathrm{x}_{2}=\mathrm{C}_{1}$

$$
4 x_{1}+x_{2}=C_{2}
$$

The given equations can be expressed as

$$
\left[\begin{array}{ll}
3 & 2 \\
4 & 1
\end{array}\right]\left[\begin{array}{l}
\mathrm{x}_{1} \\
\mathrm{x}_{2}
\end{array}\right]=\left[\begin{array}{l}
\mathrm{C}_{1} \\
\mathrm{C}_{2}
\end{array}\right]
$$

Where $\mathrm{A}=\left[\begin{array}{ll}3 & 2 \\ 4 & 1\end{array}\right], \mathrm{X}=\left[\begin{array}{l}\mathrm{x}_{1} \\ \mathrm{x}_{2}\end{array}\right], \mathrm{B}=\left[\begin{array}{l}\mathrm{C}_{1} \\ \mathrm{C}_{2}\end{array}\right]$
The characteristic equation of matrix $A$ is

$$
\begin{aligned}
|\mathrm{A}-\lambda \mathrm{I}| & =0 \\
\Rightarrow & \left|\begin{array}{cc}
3-\lambda & 2 \\
4 & 1-\lambda
\end{array}\right|=0 \\
\Rightarrow & \lambda^{2}-4 \lambda-5=0
\end{aligned}
$$

20. The plate load test was conducted on a clayey strata by using a plate of $0.3 \mathrm{~m} \times 0.3 \mathrm{~m}$ dimensions and the ultimate load per unit area for the plate was found to be 180 kPa . The ultimate bearing capacity (in kPa ) of a 2 m wide square footing would be
(A) 27
(B) 180
(C) 1200
(D) 2000
21. Ans: (B)

Sol: For clayey soils, $\mathrm{q}_{\mathrm{F}}=\mathrm{q}_{\mathrm{P}}$
(i.e. For clayey soil, the bearing capacity is independent of the width.)
21. If a centrifugal pump has an impeller speed of N (in rpm), discharge Q (in $\mathrm{m}^{3} / \mathrm{s}$ ) and the total head H (in m), the expression for the specific speed $\mathrm{N}_{\mathrm{s}}$ of the pump is given by
(A) $\mathrm{N}_{\mathrm{s}}=\frac{\mathrm{NQ}^{0.5}}{\mathrm{H}^{0.5}}$
(B) $\mathrm{N}_{\mathrm{s}}=\frac{\mathrm{NQ}^{0.5}}{\mathrm{H}}$
(C) $\mathrm{N}_{\mathrm{s}}=\frac{\mathrm{NQ}^{0.5}}{\mathrm{H}^{0.75}}$
(D) $\mathrm{N}_{\mathrm{s}}=\frac{\mathrm{NQ}}{\mathrm{H}^{0.75}}$
21. Ans: (C)

## Sol:

Specific speed of a centrifugal pump
$N_{S P}=\frac{N \sqrt{Q}}{(H)^{3 / 4}}=\frac{N \cdot Q^{0.5}}{H^{0.75}}$

Where
$\mathrm{N}=$ speed of an impeller (rpm)
$\mathrm{Q}=$ Discharge ( $\mathrm{m}^{3} / \mathrm{sec}$ )
$\mathrm{H}=$ Total head developed (m)
22. The divergence of the vector field $V=x^{2} i+2 y^{3} j+z^{4} k$ at $x=1, y=2, z=3$ is $\qquad$
22. Ans: 134

Sol:

$$
\begin{aligned}
\vec{V} & =x^{2} \dot{i}+2 y^{3} \dot{j}+z^{4} \vec{k} \\
\operatorname{div} \vec{V} & =2 x+6 y^{2}+4 z^{3} \\
\text { at } x & =1, y=2, z=3, \\
\operatorname{div} \vec{V} & =2(1)+6(4)+4(27)=134
\end{aligned}
$$

23. The safety within a roundabout and the efficiency of a roundabout can be increased, respectively by.
(A) Increasing the entry radius and increasing the exit radius
(B) increasing the entry radius and decreasing the exit radius
(C) Decreasing the entry radius and increasing the exit radius
(D) Decreasing the entry radius and decreasing the exit radius
24. Ans: (C)

Sol:
The safety of round about can be achieved by decreasing entry radius (decreases speed at entry) and increasing exit radius (increasing exit speed)
24. Consider the following statements related to the pore pressure parameters, A and B
P. A always lies between 0 and 1.0
Q. A can be less than 0 or greater than 1.0
R. B always lies between 0 and 1.0
S. B can be less than 0 or greater than 1.0

For these statements, which one of the following options is correct?
(A) P and R
(B) P and S
(C) Q and R
(D) Q and S
24. Ans: (C)

Sol: $0 \leq \mathrm{B} \leq 1$
A can be $<0$ (i.e. A can be - ve) for heavily OC clays and dense sands

25. The most important type of species involved in the degradation of organic matter in the case of activated sludge process is
(A) autotrophs
(B) heterotrophs
(C) prototrophs
(D) photo-autotrophs
25. Ans: (B)

Sol: ASP host heterotrophic aerobic organisms.
26. Consider the three prismatic beams with the clamped supports $P, Q$ and $R$ as shown in the figures.


Given that the modulus of elasticity, E is $2.5 \times 10^{4} \mathrm{MPa}$; and the moment of inertia, I is $8 \times 10^{3}$ $\mathrm{mm}^{4}$, the correct comparison of magnitudes of the shear force $S$ and the bending moment $M$ developed at the supports is
(A) $\mathrm{S}_{\mathrm{P}}<\mathrm{S}_{\mathrm{Q}}<\mathrm{S}_{\mathrm{R}} ; \mathrm{M}_{\mathrm{p}}=\mathrm{M}_{\mathrm{Q}}=\mathrm{M}_{\mathrm{R}}$
(B) $\mathrm{S}_{\mathrm{P}}=\mathrm{S}_{\mathrm{Q}}>\mathrm{S}_{\mathrm{R}} ; \mathrm{M}_{\mathrm{P}}=\mathrm{M}_{\mathrm{Q}}>\mathrm{M}_{\mathrm{R}}$
(C) $\mathrm{S}_{\mathrm{F}}<\mathrm{S}_{\mathrm{Q}}>\mathrm{S}_{\mathrm{R}}=\mathrm{M}_{\mathrm{P}}=\mathrm{M}_{\mathrm{Q}}=\mathrm{M}_{\mathrm{R}}$
(D) $\mathrm{S}_{\mathrm{P}}<\mathrm{S}_{\mathrm{Q}}<\mathrm{S}_{\mathrm{R}} ; \mathrm{M}_{\mathrm{F}}<\mathrm{M}_{\mathrm{Q}}<\mathrm{M}_{\mathrm{R}}$

## 26. Ans: (C)

Sol: Bending moments

$$
\begin{aligned}
& M_{P}=80 \times 8=640 \mathrm{kN}-\mathrm{m} \\
& M_{Q}=20 \times 8 \times 4=640 \mathrm{kN}-\mathrm{m} \\
& M_{R}=640 \mathrm{kN}-\mathrm{m}
\end{aligned}
$$

$$
\therefore \mathrm{M}_{\mathrm{P}}=\mathrm{M}_{\mathrm{Q}}=\mathrm{M}_{\mathrm{R}}
$$

Shear force values
$\mathrm{S}_{\mathrm{P}}=80 \mathrm{kN}$
$\mathrm{S}_{\mathrm{Q}}=20 \times 8=160 \mathrm{kN}$
$S_{\mathrm{R}}=0$

$$
\therefore \mathrm{S}_{\mathrm{P}}<\mathrm{S}_{\mathrm{Q}}>\mathrm{S}_{\mathrm{R}}
$$

27. A simply supported rectangular concrete beam of span 8 m has to be prestressed with a force of 1600 kN . The tendon is of parabolic profile having zero eccentricity at the supports. The beam has to carry an external uniformly distributed load of intensity $30 \mathrm{kN} / \mathrm{m}$. Neglecting the self - weight of the beam, the maximum dip (in meters, up to two decimal places) of the tendon at the mid-span to balance the external load should be $\qquad$
28. Ans: 0.15

## Sol:

## Given:

$$
\begin{aligned}
& l=8 \mathrm{~m} \\
& \mathrm{P}=1600 \mathrm{kN} \\
& \mathrm{~h}=?
\end{aligned}
$$

From the load balancing concept


$$
\begin{gathered}
\mathrm{m}_{\mathrm{E}}=\mathrm{m}_{\mathrm{p}} \\
\frac{\mathrm{w} \ell^{2}}{8}=\mathrm{Ph} \\
\mathrm{~h}=\frac{\mathrm{w} \ell^{2}}{8 \mathrm{P}}=\frac{30 \times 8^{2}}{8 \times 1600}=0.15 \mathrm{~m}
\end{gathered}
$$

28. For a given water sample, the ratio between $\mathrm{BOD}_{5 \text {-day }}, 20^{\circ} \mathrm{C}$ and the ultimate BOD is 0.68 . The value of the reaction rate constant k (on base e) (in day ${ }^{-1}$, up to two decimal places) is $\qquad$
29. Ans: 0.23

Sol: $\quad \frac{5 \text { day } \mathrm{BOD} 20^{\circ} \mathrm{C}}{\text { ultimate } \mathrm{BOD}}=\frac{\mathrm{y}_{5}^{20^{\circ} \mathrm{C}}}{\mathrm{L}_{\mathrm{o}}}=0.68$

$$
\begin{aligned}
& \frac{\mathrm{L}_{\mathrm{o}}\left[1-\mathrm{e}^{-\mathrm{k}_{20} \times 5}\right]}{\mathrm{L}_{\mathrm{o}}}=0.68 \\
& \Rightarrow \mathrm{~K}_{20}=0.2278 \mathrm{~d}^{-1}
\end{aligned}
$$

29. Two plates of 8 mm thickness each are connected by a fillet weld of 6 mm thickness as shown in the figure.


The permissible stresses in the plate and the weld are 150 MPa and 110 MPa , respectively. Assuming the length of the weld shown in the figure to be the effective length, the permissible load $P($ in $k N)$ is $\qquad$
29. Ans: 60

Sol: The permissible stress in the plate $\sigma_{\mathrm{at}}=150 \mathrm{Mpa}$
The permissible stress in the weld $\tau_{\mathrm{vf}}=110 \mathrm{Mpa}$
Strength of smaller width plate $\mathrm{P}_{\mathrm{t}}=\mathrm{A}_{\mathrm{g}} \times \sigma_{\mathrm{at}}=50 \times 8 \times 150=60 \times 10^{3} \mathrm{~N}=60 \mathrm{kN}$
$\left(\mathrm{A}_{\mathrm{g}}=\right.$ Gross sectional area of smaller width plate $\left.=50 \times 8=400 \mathrm{~mm}^{2}\right)$
(Since smaller width plate has least sectional area and least strength)
Effective length of fillet weld $\mathrm{L}_{\mathrm{w}}=2 \times 100+50=250 \mathrm{~mm}$
Size of fillet weld $S=6 \mathrm{~mm}$
Effective throat thickness $\mathrm{t}_{\mathrm{t}}=0.707 \times \mathrm{S}=0.707 \times 6=4.242 \mathrm{~mm}$
Strength of fillet weld $\mathrm{P}_{\mathrm{s}}=\mathrm{L}_{\mathrm{w}} \times \mathrm{t}_{\mathrm{t}} \times \tau_{\mathrm{vf}}=250 \times 4.242 \times 110=116.65 \times 10^{3} \mathrm{~N}=116.65 \mathrm{kN}$
The permissible load $P=$ Smaller of $P_{t}$ and $P_{s}=60 \mathrm{kN}$

## The permissible load $\mathbf{P}$ (in kN ) is 60(Answer)

30. Consider the portal frame shown in the figure and assume the modulus of elasticity, $\mathrm{E}=2.5 \times 10^{4}$ MPa and the moment of inertia, $\mathrm{I}=8 \times 10^{8} \mathrm{~mm}^{4}$ for all the members of the frame.


The rotation (in degrees, up to decimal place) at the rigid joint Q would be $\qquad$
30. Ans: 1

Sol: $\quad \mathrm{M}=4000-3300=700 \mathrm{kN}-\mathrm{m}$


$$
\mathrm{M}_{\mathrm{FQS}}=\mathrm{M}_{\mathrm{FSQ}}=\mathrm{M}_{\mathrm{FQR}}=\mathrm{M}_{\mathrm{FRQ}}=0
$$

[ Fixed end moments]
$\theta_{\mathrm{S}}=\theta_{\mathrm{R}}=0 \quad \rightarrow$ Fixed supports

$$
\begin{array}{ll}
\mathrm{M}_{\mathrm{QS}}=\mathrm{M}_{\mathrm{FQS}}+\frac{2 \mathrm{EI}}{4}\left[2 \theta_{\mathrm{Q}}+\theta_{\mathrm{S}}\right] & \\
\mathrm{M}_{\mathrm{QS}}=\mathrm{EI} \theta_{\mathrm{Q}} & \mathrm{E}=2.5 \times 10^{4} \times 10^{3} \mathrm{kN} / \mathrm{m}^{2}
\end{array}
$$

$$
\mathrm{I}=\frac{1}{1250} \mathrm{~m}^{4}
$$

$$
\mathrm{M}_{\mathrm{QR}}=\mathrm{M}_{\mathrm{FQR}}+\frac{2 \mathrm{EI}}{4}\left[2 \theta_{\mathrm{Q}}+\theta_{\mathrm{R}}\right]
$$

$$
\begin{gathered}
\mathrm{M}_{\mathrm{QR}}=\mathrm{EI} \theta_{\mathrm{Q}} \\
\mathrm{M}_{\mathrm{QR}}+\mathrm{M}_{\mathrm{QS}} \neq 700=0 \\
\mathrm{EI} \theta_{\mathrm{Q}}+\mathrm{EI} \theta_{\mathrm{Q}}=-700 \\
\mathrm{EI} \theta_{\mathrm{Q}}=-350 \\
\theta_{\mathrm{Q}}=\frac{-350}{2.5 \times 10^{4} \times 10^{3} \times \frac{1}{1250}}=-1.75 \times 10^{-2} \text { radians }=-1.75 \times 10^{-2} \times \frac{180}{\pi}=1.002 \simeq 1
\end{gathered}
$$

31. A 1 m wide rectangular channel carries a discharge of $2 \mathrm{~m}^{3} / \mathrm{s}$. The specific energy -depth diagram is prepared for the channel. It is observed in this diagram that corresponding to a particular specific energy; the subcritical depth is twice the supercritical depth. The subcritical depth (in meters, up to two decimal places) is equal to
32. Ans: 1.07

Sol: Given
$\mathrm{Q}=2 \mathrm{~m}^{3} / \mathrm{s}$
$B=4 \mathrm{~m}$


Given that at a particular specific energy. The sub critical depth is twice the supercritical depth.

$$
\begin{gathered}
\mathrm{y}_{1}=2 \mathrm{y}_{2} \\
\mathrm{y}_{1}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}=\mathrm{y}_{2}+\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}} \\
\mathrm{y}_{1}+\frac{\mathrm{Q}^{2}}{2 \mathrm{~g}\left(\text { B. } \mathrm{y}_{1}\right)^{2}}=\mathrm{y}_{2}+\frac{\mathrm{Q}^{2}}{2 \mathrm{~g}\left(\text { B. } \mathrm{y}_{2}\right)^{2}}
\end{gathered}
$$

$$
\begin{array}{r}
\mathrm{y}_{1}+\frac{\mathrm{Q}^{2}}{2 \mathrm{y}\left(\mathrm{~B} \cdot \mathrm{y}_{1}\right)^{2}}=\frac{\mathrm{y}_{1}}{2}+\frac{\mathrm{Q}^{2}}{2 \mathrm{~g}\left(\mathrm{~B} \cdot \frac{\mathrm{y}_{1}}{2}\right)^{2}} \\
\mathrm{y}_{1}+\frac{\mathrm{Q}^{2}}{2 \mathrm{~g} \cdot \mathrm{~B}^{2} \cdot \mathrm{y}_{1}^{2}}=\frac{\mathrm{y}_{1}}{2}+\frac{2 \mathrm{Q}^{2}}{\mathrm{gB}^{2} \mathrm{y}_{1}^{2}} \\
\frac{\mathrm{y}_{1}}{2}=\frac{2 \mathrm{Q}^{2}}{\mathrm{gB}^{2} \mathrm{y}_{1}^{2}}-\frac{\mathrm{Q}^{2}}{2 \mathrm{gB}^{2} \mathrm{y}_{1}^{2}} \\
\frac{\mathrm{y}_{1}}{2}=\frac{3}{2}\left(\frac{\mathrm{Q}^{2}}{\mathrm{gB}^{2} y_{1}^{2}}\right) \\
\mathrm{y}_{1}^{3}
\end{array}=\frac{3 \mathrm{Q}^{2}}{\mathrm{gB}{ }^{2}} \mathrm{y}_{1}=\sqrt[3]{\frac{3 \mathrm{Q}^{2}}{\mathrm{gB}^{2}}=\sqrt[3]{\frac{3 \times 2^{2}}{9.81 \times 1^{2}}}} \begin{gathered}
\mathrm{y}_{1}=1.069 \approx 1.07
\end{gathered}
$$

32. Following are the statements related to the stress path in a triaxial testing of soils:
P. If $\sigma_{1}=\sigma_{3}$, the stress point lies at the origin of the $\mathrm{p}-\mathrm{q}$ plot.
Q. If $\sigma_{1}=\sigma_{3}$, the stress point lies on the $p$-axis of the $p-q$ plot.
R. If $\sigma_{1}>\sigma_{3}$, both the stress points $p$ and $q$ are positive.

For the above statements, the correct combination is
(A) P-False; Q - True; R - True
(B) P-True Q - False; R - True
(C) P - False; Q - True; R - False
(D) P-True; Q - False; R - False
32. Ans: (A)

Sol:

$$
\xrightarrow[\mathrm{p}]{\stackrel{\left(\frac{\sigma_{1}-\sigma_{3}}{2}\right)}{\xrightarrow[\text { Stress path }]{ }}\left(\frac{\sigma_{1}-\sigma_{3}}{2}\right)}
$$

33. The composition of a municipal solid waste sample is given below:

| Component | Percent by Mass | Moisture Content (\%) | Energy content (kJ/kg, on as <br> -discarded basis) |
| :--- | :---: | :---: | :---: |
| Food waste | 20 | 70 | 2500 |
| Paper | 10 | 4 | 10000 |
| Cardboard | 10 | 4 | 8000 |
| Plastics | 10 | 1 | 14000 |
| Garden <br> Trimmings | 40 | 60 | 3500 |
| Wood | 5 | 20 | 14000 |
| Tin Cans | 5 | 2 | 100 |

The difference between the energy content of the waste sample calculated on dry basis and asdiscarded basis (in $\mathrm{kJ} / \mathrm{kg}$ ) would be $\qquad$
33. Ans: 3870

Sol: Total energy as discarded

$$
=\frac{\sum \mathrm{P}_{\mathrm{i}} \mathrm{E}_{\mathrm{i}}}{100}=\frac{\mathrm{P}_{1} \mathrm{E}_{1}+\mathrm{P}_{2} \mathrm{E}_{2}+\ldots \ldots \ldots+\mathrm{P}_{\mathrm{n}} \mathrm{E}_{\mathrm{n}}}{100}
$$

$$
\begin{aligned}
& {[20 \times 2500+10 \times 10000+10 \times 8000+} \\
= & \frac{10 \times 14000+40 \times 3500+5 \times 14000+5 \times 100]}{100} \\
= & 5805 \mathrm{~kJ} / \mathrm{kg}
\end{aligned}
$$

Moisture content of MSW

$$
\begin{aligned}
& =\frac{20 \times 70+10 \times 4+10 \times 4+10 \times 1+40 \times 60+5 \times 20 \times 2+5 \times 2}{100} \\
& =40 \%
\end{aligned}
$$

Total energy on dry basis

$$
\begin{aligned}
& =\text { energy as discarded } \times \frac{100}{100-\% \mathrm{mc}} \\
& =5805 \times \frac{100}{100-40}=9675 \mathrm{~kJ} / \mathrm{kg}
\end{aligned}
$$

Difference in energy content $=9675-5805$

$$
=3870 \mathrm{~kJ} / \mathrm{kg}
$$

34. Consider the following second -order differential equation:

$$
y^{\prime \prime}-4 y^{\prime}+3 y=2 t-3 t^{2}
$$

The particular solution of the differential equation is
(A) $-2-2 t-t^{2}$
(B) $-2 t-t^{2}$
(C) $2 \mathrm{t}-3 \mathrm{t}^{2}$
(D) $-2-2 t-3 t^{2}$
34. Ans: (A)

Sol:

$$
\begin{aligned}
& y^{11}-4 y^{1}+3 y=2 t-3 t^{2} \\
& \left(D^{2}-4 D+3\right) y=2 t-3 t^{2} \\
& P I=\frac{1}{\left(D^{2}-4 D+3\right)}\left(2 t-3 t^{2}\right) \\
& \quad=\frac{1}{3\left[1+\left(\frac{D^{2}-4 D}{3}\right)\right]}\left(2 t-3 t^{2}\right)
\end{aligned}
$$

$$
\begin{aligned}
& =\frac{1}{3}\left\{1+\left(\frac{D^{2}-4 D}{3}\right)\right\}^{-1}\left(2 t-3 t^{2}\right) \\
& =\frac{1}{3}\left\{1-\left(\frac{D^{2}-4 D}{3}\right)+\left(\frac{D^{2}-4 D}{3}\right)^{2}\right\}\left(2 t-3 t^{2}\right)
\end{aligned}
$$

(Expanding by binomial theorem up to $\mathrm{D}^{2}$ terms)

$$
\begin{aligned}
& =\frac{1}{3}\left\{\left(2 \mathrm{t}-3 \mathrm{t}^{2}\right)-\left[\frac{-6-4(2-6 \mathrm{t})}{3}\right]+\frac{16}{9}(-6)\right\} \\
& =-2-2 \mathrm{t}-\mathrm{t}^{2}
\end{aligned}
$$

35. A municipal corporation is required to treat $1000 \mathrm{~m}^{3} /$ day of water. If is found that an overflow rate of 20 m /day will produce a satisfactory removal of the discrete suspended particles at a depth of 3 m . The diameter (in meters, rounded to the nearest integer) of a circular settling tank designed for the removal of these particles would be $\qquad$
36. Ans: 8

Sol: $\quad \mathrm{Q}=1000 \mathrm{~m}^{3} /$ day
SOR $=20 \mathrm{~m} /$ day
Diameter of settling tank $d=$ ?
Surface area of settling tank $=\frac{\mathrm{Q}}{\mathrm{SOR}}$

$$
=\frac{1000}{20}=50 \mathrm{~m}^{2}
$$

$$
\begin{array}{r}
\frac{\pi}{4} d^{2}=50 \\
d=\sqrt{\frac{50 \times 4}{\pi}}=7.978 \mathrm{~m}
\end{array}
$$

36. The figure shows a U-tube having a $5 \mathrm{~mm} \times 5 \mathrm{~mm}$ square cross-section filled with mercury (specific gravity $=13.6$ ) up to a height of 20 cm in each limb (open to the atmosphere).


If $5 \mathrm{~cm}^{3}$ of water is added to the right limb, the new height (in cm , up to two decimal places) of mercury in the LEFT limb will be $\qquad$
36. Ans: 20.74

## Sol:

Volume of water, $\mathrm{V}=5 \mathrm{~cm}^{3}$
$\mathrm{A} \times \mathrm{h}_{\mathrm{w}}=5$
$(0.5 \times 0.5) \times \mathrm{h}_{\mathrm{w}}=5$
$h_{w}=20 \mathrm{~cm}$
By the manometry principle
Pressure in left limb = pressure in right limb

$\mathrm{P}_{\mathrm{x}}=\mathrm{P}_{\mathrm{y}}$
$\rho_{\mathrm{w}} \times \mathrm{g} \times \mathrm{h}_{\mathrm{w}}=\rho_{\mathrm{Hg}} \times \mathrm{g} \times \mathrm{h}_{\mathrm{Hg}}$
$\mathrm{S}_{\mathrm{w}} \times \mathrm{h}_{\mathrm{w}}=\mathrm{S}_{\mathrm{Hg}} \times \mathrm{h}_{\mathrm{Hg}}$
$1 \times 20=13.6 \times(2 h)$
$\mathrm{h}=0.735$
New height $=$ initial mercury height +h
$=20+0.735$
$=20.735 \approx 20.74$
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37. Water is pumped at a steady uniform flow rate of $0.01 \mathrm{~m}^{3} / \mathrm{s}$ through a horizontal smooth circular pipe of 100 mm diameter. Given that the Reynolds number is 800 and g is $9.81 \mathrm{~m} / \mathrm{s}^{2}$, the head loss (in meters, up to one decimal place) per km length due to friction would be $\qquad$
37. Ans: 66.1

Sol:

$$
\mathrm{h}_{\mathrm{L}}=\frac{\mathrm{f} \ell \mathrm{v}^{2}}{2 \mathrm{gd}}=\frac{\mathrm{f} \ell \mathrm{Q}^{2}}{12.1 \times \mathrm{d}^{5}}
$$

For laminar flow ( $\mathrm{Re}<2000$ ),

$$
\begin{aligned}
\mathrm{f} & =\frac{64}{\mathrm{Re}}=\frac{64}{800}=0.08 \\
\mathrm{~h}_{\mathrm{L}} & =\frac{0.08 \times 1000 \times(0.01)^{2}}{12.1 \times(0.1)^{5}} \\
& =66.1 \mathrm{~m}
\end{aligned}
$$

38. The analysis of a water sample produces the following results:

| Ion | Milligram per <br> milli-equivalent <br> for the ion | Concentrati <br> on <br> $(\mathrm{mg} / \mathrm{L})$ |
| :--- | :--- | :--- |
| $\mathrm{Ca}^{2+}$ | 20.0 | 60 |
| $\mathrm{Mg}^{2+}$ | 12.2 | 36.6 |
| $\mathrm{Na}^{+}$ | 23.0 | 92 |
| $\mathrm{~K}^{+}$ | 39.1 | 78.2 |
| $\mathrm{Cl}^{-}$ | 35.5 | 71 |
| $\mathrm{SO}_{4}{ }^{2-}$ | 48.0 | 72 |
| $\mathrm{HCO}_{3}{ }^{-}$ | 61.0 | 122 |

The total hardness (in $\mathrm{mg} / \mathrm{L}$ as $\mathrm{CaCO}_{3}$ ) of the water sample is $\qquad$
38. Ans: 300

Sol: Total hardness $=\mathrm{Ca}^{2+} \times \frac{50}{20}+\mathrm{Mg}^{2+} \times \frac{50}{12.2}$

$$
\begin{aligned}
& =60 \times \frac{50}{20}+36.6 \times \frac{50}{12.2} \\
& =300 \mathrm{mg} / \mathrm{l} \text { as } \mathrm{CaCO}_{3}
\end{aligned}
$$

39. Consider a square - shaped area ABCD on the ground with its centre at M as shown in the figure. Four concentrated vertical loads of $\mathrm{P}=5000 \mathrm{kN}$ are applied on this area, one at each corner.


The vertical stress increment (in kPa , up to one decimal place) due to these loads according to the Boussinesq's equation, at a point 5 m right below M . is

## 39. Ans: 190.8

Sol: Concentrated load $\mathrm{P}=5000 \mathrm{kN}$
Depth $Z=5 \mathrm{~m}$
Radial distance, $r=\sqrt{x^{2}+y^{2}}$

$$
=\sqrt{2^{2}+2^{2}}=2.828 \mathrm{~m}
$$

In total, there are four concentrated loads and the stress at point M due to each load $(\mathrm{P})$ is equal.
$\therefore$ Total stress, $\sigma_{z}=4 \times \frac{\mathrm{P}}{\mathrm{z}^{2}} \cdot \frac{3}{2 \pi}\left[\frac{1}{1+\left(\frac{\mathrm{r}}{\mathrm{z}}\right)}\right]^{5 / 2}$

$$
\begin{aligned}
& =4 \times \frac{5000}{5^{2}} \times \frac{3}{2 \pi}\left[\frac{1}{1+\left(\frac{2.828}{5}\right)^{2}}\right]^{5 / 2} \\
& =190.84 \mathrm{kPa}
\end{aligned}
$$

40. Two prismatic beams having the same flexural rigidity of $1000 \mathrm{kN}-\mathrm{m}^{2}$ are shown in the figures.


If the mid-span deflections of these beams are denoted by $\delta_{1}$ and $\delta_{2}$ (as indicated in the figures), the correct option is
(A) $\delta_{1}=\delta_{2}$
(B) $\delta_{1}<\delta_{2}$
(C) $\delta_{1}>\delta_{2}$
(D) $\delta_{1} \gg \delta_{2}$
40. Ans: (A)

## Sol:

Central deflection of beam 1

$$
\delta_{1}=\frac{5 \times 6 \times 4^{4}}{384 \mathrm{EI}}=\frac{20}{\mathrm{EI}}
$$

Central deflection of beam 2

$$
\begin{aligned}
& \delta_{2}=\frac{120(2)^{3}}{48 \mathrm{EI}}=\frac{20}{\mathrm{EI}} \\
& \delta_{1}=\delta_{2}
\end{aligned}
$$

41. If $\mathrm{A}=\left[\begin{array}{ll}1 & 5 \\ 6 & 2\end{array}\right]$ and $\mathrm{B}=\left[\begin{array}{ll}3 & 7 \\ 8 & 4\end{array}\right] \mathrm{AB}^{\mathrm{T}}$ is equal to
(A) $\left[\begin{array}{ll}38 & 28 \\ 32 & 56\end{array}\right]$
(b) $\left[\begin{array}{cc}3 & 40 \\ 42 & 8\end{array}\right]$
(c) $\left[\begin{array}{ll}43 & 27 \\ 34 & 50\end{array}\right]$
(D) $\left[\begin{array}{ll}38 & 32 \\ 28 & 56\end{array}\right]$
42. Ans: (A)

Sol:

$$
\begin{aligned}
& \mathrm{A}=\left[\begin{array}{ll}
1 & 5 \\
6 & 2
\end{array}\right] \quad \mathrm{B}=\left[\begin{array}{ll}
3 & 7 \\
8 & 4
\end{array}\right], \mathrm{B}^{\mathrm{T}}=\left[\begin{array}{ll}
3 & 8 \\
7 & 4
\end{array}\right] \\
& \mathrm{AB}^{\mathrm{T}}=\left[\begin{array}{ll}
1 & 5 \\
6 & 2
\end{array}\right]\left[\begin{array}{ll}
3 & 8 \\
7 & 4
\end{array}\right]=\left[\begin{array}{cc}
3+35 & 8+20 \\
18+14 & 48+8
\end{array}\right] \\
&=\left[\begin{array}{ll}
38 & 28 \\
32 & 56
\end{array}\right]
\end{aligned}
$$

42. Two cars P and Q are moving in a racing track continuously for two hours. Assume that no other vehicles are using the track during this time. The expressions relating the distance travelled d (in km ) and time t (in hour) for both the vehicles are given as

P: $d=60 t$
Q: $d=60 t^{2}$
Within the first one hour, the maximum space headway would be
(A) 15 km at 30 minutes
(B) 15 km at 15 minutes
(C) 30 km at 30 minutes
(D) 30 km at 15 minutes
42. Ans: (A)

Sol: Two cars P and Q are moving continuously for 'two' hours
For $\mathrm{P}: \mathrm{d}=60 \mathrm{t}$
For $\mathrm{Q}: \mathrm{d}=60 \mathrm{t}^{2}$
With $\frac{1}{2}$ hour the distance travelled are

$$
\text { For } \mathrm{P}: \mathrm{d}=60 \times \frac{1}{2}=30 \mathrm{~km}
$$

$$
\text { For Q: } \mathrm{d}=60 \times\left(\frac{1}{2}\right)^{2}=\frac{60}{4}=15 \mathrm{~km}
$$

Spacing at $30 \mathrm{~min}=15 \mathrm{~km}$
43. The tangent to the curve represented by $y=x \ln x$ is required to have $45^{\circ}$ inclination with the $x$ axis. The coordinates of the tangent point would be
(A) $(1,0)$
(B) $(0,1)$
(C) $(1,1)$
(D) $(\sqrt{2}, \sqrt{2})$
43. Ans: (A)

Sol:

$$
\begin{gathered}
y=x \log _{e} x \\
\frac{d y}{d x}=x\left(\frac{1}{x}\right)+\left(\log _{e} x\right) 1=1+\log _{e} x \\
\Rightarrow \tan 45^{\circ}=1+\log _{e} x \\
\Rightarrow 1=1+\log _{e} x \\
\Rightarrow \log _{e} x=0 \\
\Rightarrow x=e^{0}=1 \\
x=1 \Rightarrow y=0 \quad\left(\because y=x \log _{e} x\right) \\
\therefore \text { The point is }(1,0)
\end{gathered}
$$

44. A catchment is idealized as a $25 \mathrm{~km} \times 25 \mathrm{~km}$ square. It has five rain gauges, one at each corner and one at the center, as shown in the figure.


During a month, the precipitation at these gauges is measured as $\mathrm{G}_{1}=300 \mathrm{~mm}, \mathrm{G}_{2}=285 \mathrm{~mm}, \mathrm{G}_{3}=$ $272 \mathrm{~mm}, \mathrm{G}_{4}=290 \mathrm{~mm}$ and $\mathrm{G}_{5}=288 \mathrm{~mm}$. The average precipitation (in mm , up to one decimal place) over the catchment during this month by using the Thiessen polygon method is $\qquad$
44. Ans: 287.4 mm

Sol:

$$
\begin{aligned}
\mathrm{G}_{1} & =300 \mathrm{~mm} \\
\mathrm{G}_{2} & =285 \mathrm{~mm} \\
\mathrm{G}_{3} & =272 \mathrm{~mm} \\
\mathrm{G}_{4} & =290 \mathrm{~mm} \\
\mathrm{G}_{5} & =288 \mathrm{~mm}
\end{aligned}
$$



$$
\begin{aligned}
& \begin{array}{l}
\mathrm{A}_{1}=\mathrm{A}_{2}=\mathrm{A}_{3}=\mathrm{A}_{4}=\frac{1}{2} \times 12.5 \times 12.5 \\
\quad=78.125 \mathrm{~km}^{2} \\
\mathrm{~A}_{5}=17.67 \times 17.67=312.5 \mathrm{~km}^{2} \\
\overline{\mathrm{P}}=\frac{78.125[300+285+272+290]+312.5 \times 288}{(25 \times 25)}
\end{array}
\end{aligned}
$$

$=287.375 \mathrm{~mm}$
45. A hollow circular shaft has an outer diameter of 100 mm and inner diameter of 50 mm . If the allowable shear stress is 125 MPa , the maximum torque (in $\mathrm{kN}-\mathrm{m}$ ) that the shaft can resist is $\qquad$
45. Ans: 23 kNm

Sol:
The maximum allowable shear stress on shaft, $\tau_{\mathrm{m}}=125 \mathrm{MPa}$
Outer diameter, $\mathrm{D}=100 \mathrm{~mm}$
Inner diameter, $\mathrm{d}=50 \mathrm{~mm}$
Torque allowed on the shaft

$$
\begin{aligned}
T & =\tau_{\mathrm{m}} \mathrm{Z}_{\mathrm{P}} \\
& =\tau_{\mathrm{m}}\left[\frac{\pi}{16 \mathrm{D}}\left(\mathrm{D}^{4}-\mathrm{d}^{4}\right)\right] \\
& =125\left[\frac{\pi}{16 \times 100}\left(100^{4}-50^{4}\right)\right] \\
& =23 \times 10^{6} \mathrm{~N}-\mathrm{mm} \\
& =23 \mathrm{kN}-\mathrm{m}
\end{aligned}
$$

46. An observer standing on the deck of a ship just sees the top of a lighthouse. The top of the lighthouse is 40 m above the sea level and the height of the observer's eye is 5 m above the sea level. The distance (in km, up to one decimal place) of the observer from the lighthouse is $\qquad$
47. Ans: 32.9 km

Sol: Distance to visible horizon $=\mathrm{D}=\sqrt{\frac{\mathrm{h}}{0.06735}}$

$$
\begin{aligned}
& \text { 'h' 'm'\& 'D' } \Rightarrow \text { 'km' } \\
& \begin{aligned}
\therefore \mathrm{D}=\mathrm{D}_{1}+\mathrm{D}_{2}
\end{aligned} \\
& \\
& \\
& =\sqrt{\frac{40}{0.06735}}+\sqrt{\frac{5}{0.06735}} \\
& \\
& =24.370+8.616 \\
& \\
&
\end{aligned}
$$

47. For the construction of a highway, a cut is to be made as shown in the figure.


The soil exhibit $\mathrm{c}^{\prime}=20 \mathrm{kPa}, \phi^{\prime}=18^{\circ}$, and the undrained shear strength $=80 \mathrm{kPa}$. The unit weight of water is $9.81 \mathrm{kN} / \mathrm{m}^{3}$. The unit weights of the soil above and below the ground water table are 18 and $20 \mathrm{kN} / \mathrm{m}^{3}$, respectively. If the shear stress at Point A is 50 kPa , the factors of safety against the shear failure at this point, considering the undrained and drained conditions, respectively, would be
(A) 1.6 and 0.9
(B) 0.9 and 1.6
(C) 0.6 and 1.2
(D) 1.2 and 0.6
47. Ans: (A)

Sol: The F.O. safety against shear failure, $F=\frac{S}{\tau}$

## Given:

$$
\tau=50 \mathrm{kPa}, \mathrm{C}^{\prime}=20 \mathrm{kPa}, \phi^{\prime}=18^{\circ}, \mathrm{S}_{\mathrm{u}}=80 \mathrm{kPa}
$$

At point A, the total vertical stress, $\sigma=2 \gamma+4 \gamma_{\text {sat }}$

$$
\sigma=2 \times 18+4 \times 20=116 \mathrm{kPa}
$$

At point A, the effective vertical stress, $\sigma^{\prime}=2 \gamma+4 \gamma^{\prime}$

$$
\begin{aligned}
\sigma^{\prime} & =2 \times \gamma+4\left[\gamma_{\mathrm{sat}}-\gamma_{\mathrm{w}}\right] \\
& =2 \times 18+4[20-9.81]=76.76 \mathrm{kPa}
\end{aligned}
$$

Considering undrained condition, $\mathrm{F}=\frac{\mathrm{S}_{\mathrm{u}}}{\tau}=\frac{80}{50}=1.6$
Considering drained condition, $F=\frac{S}{\tau}=\frac{C^{\prime}+\sigma^{\prime} \tan \phi^{\prime}}{1 \cap \mathrm{C}}$

$$
\mathrm{F}=\frac{20+76.76 \tan 18^{\circ}}{50}=0.9
$$

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48. The radii of relative stiffness of the rigid pavements P and Q are denoted by $l_{\mathrm{p}}$ and $l_{\mathrm{Q}}$. respectively.

The geometric and material properties of the concrete slab and underlying soil are given below:

| Pavement | Concrete |  |  |  |  | Soil |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Length of Slab | Breadth of Slab | Thickness of Slab | Modulus of Elasticity | Poisson's <br> Ratio | Subgrade <br> Reaction <br> Modulus |
| P | L | B | h | E | $\mu$ | K |
| Q | L | B | 0.5h | E | $\mu$ | 2K |

The ratio (up to one decimal place ) of $\frac{\ell_{\mathrm{p}}}{\ell_{\mathrm{Q}}}$ is
48. Ans: 2

## Sol:

Westergaard defined the term radius of relative stiffness, $l$ which is expressed as

$$
\begin{aligned}
\ell & =\left[\frac{E^{3}}{12 \mathrm{k}\left(1-\mu^{2}\right)}\right]^{1 / 4} \\
\ell_{\mathrm{P}} & =\left[\frac{\mathrm{Eh}^{3}}{12 \mathrm{k}\left(1-\mu^{2}\right)}\right]^{1 / 4} \\
\ell_{\mathrm{Q}} & =\left[\frac{\mathrm{E}(0.5 \mathrm{~h})^{3}}{12(2 \mathrm{k})\left(1-\mu^{2}\right)}\right]^{1 / 4} \\
& =\left[\frac{(0.5)^{3}}{2} \times \frac{\mathrm{Eh}^{3}}{12 \mathrm{k}\left(1-\mu^{2}\right)}\right]^{1 / 4} \\
\left(\frac{(0.5)^{3}}{2}\right)^{1 / 4} & {\left[\frac{E h^{3}}{12 \mathrm{k}\left(1-\mu^{2}\right)}\right]^{1 / 4} }
\end{aligned}
$$

$$
\begin{aligned}
\frac{\ell_{\mathrm{P}}}{\ell_{\mathrm{Q}}} & =\frac{\left[\frac{\mathrm{Eh}^{3}}{12 \mathrm{k}\left(1-\mu^{2}\right)}\right]^{1 / 4}}{\left(\frac{(0.5)^{3}}{2}\right)^{1 / 4}\left[\frac{\mathrm{Eh}^{3}}{12 \mathrm{k}\left(1-\mu^{2}\right)}\right]^{1 / 4}} \\
& =\left(\frac{2}{(0.5)^{3}}\right)^{1 / 4} \\
\frac{\ell_{\mathrm{P}}}{\ell_{\mathrm{Q}}} & =2
\end{aligned}
$$

49. Two identical concrete piles having the plan dimensions $50 \mathrm{~cm} \times 50 \mathrm{~cm}$ are driven into a homogeneous sandy layer as shown in the figures. Consider the bearing capacity factor $\mathrm{N}_{\mathrm{q}}$ for $\phi=$ $30^{\circ}$ as 24 .


If $\mathrm{Q}_{\mathrm{PI}}$ and $\mathrm{Q}_{\mathrm{P} 2}$ represent the ultimate point bearing resistances of the piles under dry and submerged conditions, respectively, which one of the following statements is correct?
(A) $\mathrm{Q}_{\mathrm{P} 1}>\mathrm{Q}_{\mathrm{P} 2}$ by about $100 \%$
(B) $\mathrm{Q}_{\mathrm{P} 1}<\mathrm{Q}_{\mathrm{P} 2}$ by about $100 \%$
(C) $\mathrm{Q}_{\mathrm{P} 1}>\mathrm{Q}_{\mathrm{P} 2}$ by about $5 \%$
(D) $\mathrm{Q}_{\mathrm{P} 1}<\mathrm{Q}_{\mathrm{P} 2}$ by about $5 \%$
49. Ans: (A)

Sol: The point bearing resistance of piles in sandy soils
$\mathrm{Q}_{\mathrm{P}}=\mathrm{A}_{\mathrm{b}} \cdot \sigma_{\mathrm{v}}^{\prime} \cdot \mathrm{N}_{\mathrm{q}}$
As the area at base $\left(\mathrm{A}_{\mathrm{b}}\right)$ and $\mathrm{N}_{\mathrm{q}}$ are same for both the piles given, $\mathrm{Q}_{\mathrm{P}} \propto \sigma_{\mathrm{v}}^{\prime}$
For dry sand condition, $\sigma_{v}^{\prime}=\sigma_{v}=20 \times \gamma$
For submerged condition, $\sigma_{v}^{\prime}=20 \gamma^{\prime}$

> Since $\gamma^{\prime} \approx \frac{1}{2}$ of $\gamma$
> $\therefore \mathrm{QP}_{2} \approx \frac{1}{2}$ of $\mathrm{QP}_{1}$
> (or) $\mathrm{QP}_{1} \approx 2 \mathrm{QP}_{2}$
(or) $\mathrm{Q}_{\mathrm{P} 1}>\mathrm{QP}_{2}$ by about $100 \%$
50. Consider the following definite integral
$I=\int_{0}^{1} \frac{\left(\sin ^{-1} x\right)^{2}}{\sqrt{1-x^{2}}} d x$
The value of the integral is
(A) $\frac{\pi^{3}}{24}$
(B) $\frac{\pi^{3}}{12}$
(C) $\frac{\pi^{3}}{48}$
(D) $\frac{\pi^{3}}{64}$
50. Ans: (A)

Sol:

$$
\begin{array}{ll}
\mathrm{I}=\int_{0}^{1} \frac{\left(\sin ^{-1} \mathrm{x}\right)^{2}}{\sqrt{1-\mathrm{x}^{2}}} \mathrm{dx} & \text { put } \sin ^{-1} \mathrm{x}=\mathrm{t} \Rightarrow \frac{1}{\sqrt{1-\mathrm{x}^{2}}} \mathrm{dx}=\mathrm{dt} \\
\mathrm{I}=\int_{0}^{\frac{\pi}{2}}(\mathrm{t})^{2} \mathrm{dt}=\left(\frac{\mathrm{t}^{3}}{3}\right)_{0}^{\frac{\pi}{2}}=\frac{\pi^{3}}{24} & \text { If } \mathrm{x}=0 \text { then } \mathrm{t}=0, \text { If } \mathrm{x}=1 \text { then } \mathrm{t}=\frac{\pi}{2}
\end{array}
$$

51. A 2 m long, axially loaded mild steel rod of 8 mm diameter exhibits the load-displacement (P- $\delta$ ) behavior as shown in figure.


Assume the yield stress of steel as 250 MPa . The complementary strain energy (in N-mm) stored in the bar up to its linear elastic behavior will be $\qquad$

## 51. Ans: 15,707.963

## Sol:

Complimentary strain energy for linear materials.


Complimentary strain energy = strain energy

$$
\begin{aligned}
& =\frac{1}{2} \sigma_{\mathrm{y}} \times \varepsilon \times \mathrm{A} \times \mathrm{L} \\
& =\frac{1}{2} \times 250 \times \frac{2.5}{2000} \times \frac{\pi}{4} \times 8^{2} \times 2000 \quad\left(\because \varepsilon=\frac{\delta \ell}{\ell}\right) \\
& =15,707.963 \mathrm{~N}-\mathrm{mm}
\end{aligned}
$$

52. Group I gives a list of test methods and test apparatus for evaluating some of the properties of ordinary Portland cement (OPC) and concrete. Group II gives the list of these properties.

## Group I

## P. Le Chatelier test

Q. Vee-Bee test
R. Blaine air permeability test
S. The Vicat apparatus

## Group - II

1. Soundness of OPC
2. Consistency and setting time of OPC
3. Consistency or workability of concrete
4. Fineness of OPC

The correct match of the items in Group I with items in Group II is
(A) P-1, Q-3, R-4, S-2
(B) P-2, Q-3, R-1, S-4
(C) $\mathrm{P}-4, \mathrm{Q}-2, \mathrm{R}-4, \mathrm{~S}-1$
(D) P-1, Q-4, R-2, S-3
52. Ans: (A)
53. Two towers, A and B, standing vertically on a horizontal ground, appear in a vertical aerial photograph as shown in the figure.


The length of the image of the tower A on the photograph is 1.5 cm and of the tower B is 2.0 cm . The distance of the top of the tower A (as shown by the arrowhead) is 4.0 cm and the distance of the top of the tower B is 6.0 cm , as measured from the principal point p of the photograph. If the height of the tower B is 80 m , the height (in meters) of the tower A is $\qquad$
53. Ans: 90

## Sol: For Town 'A'

Relief displacement $d_{A}=1.5 \mathrm{~cm} ; \mathrm{T}_{\mathrm{A}}=4 \mathrm{~cm}$

$$
\therefore \mathrm{d}=\frac{\mathrm{h} . \mathrm{r}}{\left(\mathrm{H}-\mathrm{h}_{\text {avg }}\right)}
$$

For tower 'B'

$$
\begin{aligned}
& d_{B}=2 \mathrm{~cm} ; \mathrm{r}_{\mathrm{B}}=6 \mathrm{~cm} ; \mathrm{h}_{\mathrm{B}}=80 \mathrm{~m} \\
& \mathrm{~d}_{\mathrm{B}}=\frac{\mathrm{h}_{\mathrm{B}} \cdot \mathrm{r}_{\mathrm{B}}}{\left(\mathrm{H}-\mathrm{h}_{\text {avg }}\right)} \Rightarrow\left(\mathrm{H}-\mathrm{h}_{\text {avg }}\right)=80 \times \frac{6}{2}=240 \\
& \mathrm{~d}_{\mathrm{A}}=\frac{\mathrm{h}_{\mathrm{A}} \mathrm{r}_{\mathrm{A}}}{\left(\mathrm{H}-\mathrm{h}_{\text {avg }}\right)} \Rightarrow \mathrm{h}_{\mathrm{A}}=\frac{1.5 \times 240}{4}=90 \mathrm{~m}
\end{aligned}
$$

54. The culturable command area of a canal is 10,000 ha. The area grows only two crops-rice in the Kharif season and wheat in the Rabi season. The design discharge of the canal is based on the rice requirements, which has an irrigated area of 2500 ha, base period of 150 days and delta of 130 cm . The maximum permissible irrigated area (in ha) for wheat, with a base period of 120 days and delta of 50 cm is $\qquad$
55. Ans: 5200

Sol: Rice

$$
\begin{array}{ll}
\mathrm{A}=2500 \mathrm{ha} & \mathrm{~A}=? \\
\mathrm{~B}=150 \mathrm{~d} & \mathrm{~B}=120 \mathrm{~d} \\
\Delta=130 \mathrm{~cm} & \Delta=50 \mathrm{~cm}
\end{array}
$$

$$
\mathrm{Q}=\text { constant }
$$

$$
\begin{aligned}
\mathrm{Q} & =\frac{\mathrm{A}}{\mathrm{D}} \& \mathrm{D}=8.64 \frac{\mathrm{~B}}{\Delta} \\
\mathrm{Q} & =\frac{\mathrm{A} \Delta}{8.64 \mathrm{~B}} \\
\frac{\mathrm{~A}_{1} \Delta}{\mathrm{~B}_{1}} & =\frac{\mathrm{A}_{2} \Delta_{2}}{\mathrm{~B}_{2}}
\end{aligned}
$$

$$
\begin{aligned}
\frac{2500 \times 130}{150} & =\frac{A_{2} \times 50}{120} \\
\mathrm{~A}_{2} & =130 \times 40=5200 \mathrm{ha}
\end{aligned}
$$

55. Consider the following statements:
P. Walls of one brick thick are measured in square meters
Q. Walls of one brick thick are measured in cubic meters
R. No deduction in the brickwork quantity is made for openings in walls up to $0.1 \mathrm{~m}^{2}$ area.
S. For the measurement of excavation from the borrow pit in a fairly uniform ground, deadmen are left at suitable intervals.

For the above statements, the correct option is
(A) P-False Q - True; R- False; S-True
(B) P-False Q - True; R- False; S-False
(C) P-False Q - True; R- True; S-False
(D) P-False Q - True; R- True; S-True
55. Ans: (D)

## General Aptitude

1. Two dice are thrown simulateneously. The probability that the product o the numbers appearing on the top faces of the dice is a perfect square is
(A) $1 / 9$
(B) $2 / 9$
(C) $1 / 3$
(D) $4 / 9$
2. Ans: (B)

Sol: Total chances $=6 \times 6=36$ events
Product of numbers on 2 dice have to perfect square $=$ Favourable chances
$=(1,1),(2,2),(1,4),(3,3),(4,1),(5,5),(4,4),(6,6)=8$
Probability $=\frac{\text { Favourable chances }}{\text { Total chances }}=\frac{8}{36}=\frac{2}{9}$
$\therefore$ Option (B) is correct
02. Four cards lie on a table. Each card has a number printed on one side and a colour on the other. The faces visible on the cards are 2,3 , red, and blue.

Proposition: If a card has an even value on one side, then its opposite face is red.
The cards which MUST be turned over to verify the above proposition are
(A) 2, red
(B) 2, 3, red
(C) 2, blue
(D) 2 red, blue
02. Ans: (C)

Sol: Total number of cards $=4$
Visible numbers on the cards $=2$ and 3
Visible colours on the cards $=$ red and blue
If numbers on the cards $=1,2,3$ and 4 then possible colours are blue, red, blue and red respectively.
In order to verify the proposition, we have to turn to card 2 then opposite must be red. In all options except ' C ' 2 and red are present
$\therefore$ Option ' C ' is correct.
03. What is the value of $x$ when $81 \times\left(\frac{16}{25}\right)^{x+2}+\left(\frac{3}{5}\right)^{2 x+4}=144$ ?
(A) 1
(B) -1
(C) -2
(D) Cannot be determined
03. Ans: (B)

Sol:

$$
\begin{aligned}
& 81 \times\left(\frac{16}{25}\right)^{x+2} \div\left(\frac{3}{5}\right)^{2 x+4}=144 \\
& \frac{\left(\frac{16}{25}\right)^{x+2}}{\left(\frac{3}{5}\right)^{2 x+4}}=\frac{144}{81}=\frac{(12)^{2}}{(9)^{2}}=\left(\frac{12}{9}\right)^{2}
\end{aligned}
$$

$$
\frac{\left(\frac{4}{5}\right)^{2 x+4}}{\left(\frac{3}{5}\right)^{2 x+4}}=\left(\frac{12}{9}\right)^{2}
$$

$$
\begin{aligned}
& \frac{(4)^{2 x+4}}{(5)^{2 x+4}} \times \frac{(5)^{2 x+4}}{(3)^{2 x+4}}=\left(\frac{12}{9}\right)^{2} \\
& \frac{(4)^{2 x+4}}{(3)^{2 x+4}}=\left(\frac{12}{9}\right)^{2}=\left(\frac{4}{3}\right)^{2} \\
& \quad\left(\frac{4}{3}\right)^{2 x+4}=\left(\frac{4}{3}\right)^{2} \\
& 2 x+4=2 \\
& 2 x=-2 \\
& x=-1
\end{aligned}
$$

4. The event would have been successful if you $\qquad$ able to come.
(A) are
(B) had been
(C) have been
(D) Would have been
5. Ans: (B)

Sol: Conditional tense type (3 had+V3 +would have +V3)
05. There was no doubt that their work was thorough

Which of the words below is closet in meaning to the underlined word above?
(A) pretty
(B) complete
(C) sloppy
(D) haphazard
05. Ans: (B)

Sol: Through means including every possible detail, parts or complete or absolute.
06. $P, Q, R, S, T$ and $U$ are seated around a circular table, $R$ is seated two places to the right of $Q, P$ is seated three places to the left of R.S is seated opposite U. If P and U now switch seats, which of the following must necessarily be true?
(A) P is immediately to the right of R
(B) T is immediately to the left of P
(C) T is immediately to the left of P or P is immediately to the right of Q
(D) U is immediately to the right of R or P is immediately to the left of T

## 06. Ans: (C)

Sol: From the given data, all are seated around a circular table as follows
P
Q R

S is opposite to $U$

$P$ and $U$ are switch seated means, they are interchange their places


In option (C), before interchange $T$ is immediately to the left of $P$ and After interchange $P$ is immediately to the right of Q
$\therefore$ Option ' C ' is correct
07. Budhan covers a distance of 19 km in 2 hours by cycling one fourth of the time and walking the rest. The next day he cycles (at the same speed as before) for half the time and walks the rest (at the same speed as before) and covers 26 km in 2 hours. The speed in $\mathrm{km} / \mathrm{h}$ at which Budhan walks is
(A) 1
(B) 4
(C) 5
(D) 6

## 07. Ans: (D)

Sol:


Let cycling speed $=\mathrm{C}$
Walking speed $=\mathrm{W}$
$\frac{\mathrm{C}}{2}+\frac{3 \mathrm{~W}}{2}=19$

$\mathrm{C}+\mathrm{W}=26$.
By solving equation no (i) and (ii)
$\mathrm{W}=6 \mathrm{~km} / \mathrm{hr}$
$\therefore$ The speed at which Budhan walks $=6 \mathrm{kmph}$
08. A map shows the elevation of Darjeeling, Gangtok, Kalimpong, Pelling, and Siliguri, Kalimpong is at a lower elevation than Gangtok. Pelling is at a lower elevation than Gangtok. Pelling is at a higher elevation than siliguri. Darjeeling is at a higher elevation than Gangtok.

Which of the following statements can be inferred from the paragraph above?
i. Pelling is at a higher elevation than Kalimpong
ii. Kalimpong is at a lower elevation than Darjeeling
iii. Kalimpong is at a higher elevation than Siliguri
iv. Siliguri is at lower elevation than Gangtok
(A) Only ii
(B) Only ii and iii
(C) Only ii and iv
(D) Only iii and iv
08. Ans: (C)
09. Bhaichung was observing the pattern of people entering and leaving a car service centre. There was a single window where customers were being served. He saw that people inevitably came out of the centre in the order that they went in. However, the time they spent inside seemed to vary a lot:

Some people came out in a matter of minutes while for others it took much longer.
From this, what can one conclude?
(A) The centre operates on a first-come -first-served basis, but with variable service times, depending on specific customer needs.
(B) Customers were served in an arbitarary order, since they took varying amounts of time for service completion in the centre.
(C) Since some people came out within a few minutes of entering the centre, the system is likely to operate on a last-come-first-served basis.
(D) Entering the centre early ensured that one would have shorter service times and most people attempted to do this.
09. Ans: (A)

Sol: The key sentence is "the order that they went in"
10. The points in the graph below represent the halts of a lift for durations of 1 minute, over a period of 1 hour.

Which of the following statements are correct?

i. The elevator never moves directly from any non-ground floor to another non-ground floor over the one hour period
ii. The elevator stays on the fourth floor for the longest duration over the one hour period
(A) Only I
(B) Only ii
(C) Both i and ii
(D) Neither i nor ii

## 10. Ans: (D)

Sol: (i) is in correct as it has move directly
(ii) is incorrect as it stayed for maximum duration on the ground floor

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